

# 1. Limit states, design situations, values for loads (actions), load factors and load combinations for ordinal structures

	<b>Eurocode EN 1990</b>	<b>ASCE Standard 7-10</b>	<b>CSA</b>	<b>Australian code</b>																													
Limit state	<ul style="list-style-type: none"> <li>■ Ultimate limit state (ULS)</li> <li>■ Serviceability limit state (SLS)</li> </ul>	<ul style="list-style-type: none"> <li>■ Strength limit state</li> <li>■ Serviceability limit state</li> </ul>	<ul style="list-style-type: none"> <li>■ Ultimate limit state (ULS)</li> <li>■ Fatigue limit state (FLS)</li> <li>■ Serviceability limit state (SLS)</li> </ul>																														
Design situation	<ul style="list-style-type: none"> <li>■ Persistent situation</li> <li>■ Transient situation</li> <li>■ Accidental situation</li> <li>■ Seismic design situations</li> </ul>																																
Classification of actions (loads)	<ul style="list-style-type: none"> <li>■ Permanent action, <math>G</math></li> <li>■ Variable action, <math>Q</math></li> <li>■ Accidental action, <math>A</math></li> </ul>	<ul style="list-style-type: none"> <li>■ Permanent load</li> <li>■ Variable load</li> </ul>	<ul style="list-style-type: none"> <li>■ Permanent load, <math>G</math></li> <li>■ Variable load, <math>Q</math></li> <li>■ Rare loads, <math>E</math></li> </ul>	<ul style="list-style-type: none"> <li>■ Permanent action</li> <li>■ Imposed action</li> <li>■ Wind, snow and ice &amp; earthquake action</li> </ul>																													
Values of actions (loads)	<ul style="list-style-type: none"> <li>■ <u>For permanent action</u> <ul style="list-style-type: none"> <li>- Characteristic value, <math>G_k</math></li> </ul> </li> <li>■ <u>For variable action</u> <ul style="list-style-type: none"> <li>- Characteristic value, <math>Q_k</math></li> <li>- Combination value, <math>\Psi_0 Q_k</math></li> <li>- Frequent value, <math>\Psi_1 Q_k</math></li> <li>- Quasi-permanent value, <math>\Psi_2 Q_k</math></li> </ul> </li> <li>■ <u>For accidental action</u> <ul style="list-style-type: none"> <li>- Characteristic value, <math>A_k</math></li> </ul> </li> </ul>	<ul style="list-style-type: none"> <li>■ Nominal load, <math>Q_n</math></li> </ul>	<ul style="list-style-type: none"> <li>■ Specified loads <ul style="list-style-type: none"> <li>- <u>For permanent load</u> the mean value</li> <li>- <u>For variable action</u> <ul style="list-style-type: none"> <li><math>R \geq 50</math> years (or <math>P_{AE} \leq 0.02</math>)</li> </ul> </li> <li>- <u>For rare load</u> <ul style="list-style-type: none"> <li><math>R \approx 2500</math> years (or <math>P_{AE} \approx 0.0004</math>)</li> </ul> </li> </ul> </li> <li><math>R</math> : return period</li> <li><math>P_{AE}</math> : annual exceedance probability</li> </ul>	<ul style="list-style-type: none"> <li>■ Design events for safety</li> </ul> <table border="1" style="width: 100%; text-align: center;"> <thead> <tr> <th rowspan="2">Importance Level</th> <th colspan="4">Annual probability of exceedance</th> </tr> <tr> <th>Wind Non-cyclonic</th> <th>Wind Cyclonic</th> <th>Snow</th> <th>Earthquake</th> </tr> </thead> <tbody> <tr> <td>1</td> <td>1:100</td> <td>1:200</td> <td>1:100</td> <td>1:250</td> </tr> <tr> <td>2</td> <td>1:500</td> <td>1:500</td> <td>1:150</td> <td>1:500</td> </tr> <tr> <td>3</td> <td>1:1000</td> <td>1:1000</td> <td>1:200</td> <td>1:1000</td> </tr> <tr> <td>4</td> <td>1:2000</td> <td>1:2000</td> <td>1:250</td> <td>1:1500</td> </tr> </tbody> </table>	Importance Level	Annual probability of exceedance				Wind Non-cyclonic	Wind Cyclonic	Snow	Earthquake	1	1:100	1:200	1:100	1:250	2	1:500	1:500	1:150	1:500	3	1:1000	1:1000	1:200	1:1000	4	1:2000	1:2000	1:250	1:1500
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Partial factors for actions (load factors)	<ul style="list-style-type: none"> <li>■ Partial factors for actions, <math>\gamma_f</math></li> </ul>	<ul style="list-style-type: none"> <li>■ Load factor, <math>\gamma</math></li> </ul>	<ul style="list-style-type: none"> <li>■ Load factor, <math>\alpha</math></li> </ul>	<ul style="list-style-type: none"> <li>■ None</li> </ul>																													
Design value of action (factored load)	<ul style="list-style-type: none"> <li>■ Design value (<math>F_d = \gamma_f F_r</math>)</li> </ul>	<ul style="list-style-type: none"> <li>■ Factored load, <math>\gamma Q_n</math></li> </ul>	<ul style="list-style-type: none"> <li>■ Factored load</li> </ul>	<ul style="list-style-type: none"> <li>■ Design value of action</li> </ul>																													
Combination of actions (load combinations)	<ul style="list-style-type: none"> <li>■ For example Varied by limit state and design situation</li> <li>(for example)</li> <li><math display="block">\sum_{j \geq 1} \gamma_{G,j} G_{k,j} + \gamma_P P + \gamma_{Q,1} Q_{k,1} + \sum_{i &gt; 1} \gamma_{Q,i} \Psi_{0,i} G_{k,i}</math></li> </ul>	<ul style="list-style-type: none"> <li>■ Load Combination</li> <li><math display="block">\sum_i \gamma_i (Q_n)_i</math></li> </ul>	<ul style="list-style-type: none"> <li>■ Basic Combination</li> <li><math display="block">\sum \alpha_{G_i} G_i + \alpha_{Q_j} Q_j + \sum_{k \neq j} \alpha_{C_{jk}} Q_k</math></li> <li>■ Rare Load Combination</li> <li><math display="block">\sum G_i + E + \sum_{k \neq j} \alpha_{C_{Ek}} Q_k</math></li> <li><math>\alpha_{Q_j}</math> : Principal load factor</li> <li><math>\alpha_{C_{jk}}</math> : Companion load factor</li> </ul>																														

## 2. Target reliability for ordinal structures

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Target Reliability	<p>■ Minimum values for reliability index <math>\beta</math> (ULS)</p> <table border="1"> <thead> <tr> <th rowspan="2">Reliability Class</th> <th colspan="2">reference periods</th> </tr> <tr> <th>1 year</th> <th>50 years</th> </tr> </thead> <tbody> <tr> <td>RC3</td> <td>5.2</td> <td>4.3</td> </tr> <tr> <td>RC2</td> <td>4.7</td> <td>3.8</td> </tr> <tr> <td>RC1</td> <td>4.2</td> <td>3.3</td> </tr> </tbody> </table> <p>■ Target reliability index <math>\beta</math> for Class RC2 structural members</p> <table border="1"> <thead> <tr> <th rowspan="2">Limit state</th> <th colspan="2">reference periods</th> </tr> <tr> <th>1 year</th> <th>50 years</th> </tr> </thead> <tbody> <tr> <td>USL</td> <td>4.7</td> <td>3.8</td> </tr> <tr> <td>Fatigue</td> <td colspan="2">1.5-3.8</td> </tr> <tr> <td>SLS (irreversible)</td> <td>2.9</td> <td>1.5</td> </tr> </tbody> </table>	Reliability Class	reference periods		1 year	50 years	RC3	5.2	4.3	RC2	4.7	3.8	RC1	4.2	3.3	Limit state	reference periods		1 year	50 years	USL	4.7	3.8	Fatigue	1.5-3.8		SLS (irreversible)	2.9	1.5	<p>■ Acceptable reliability (maximum annual probability of failure) associated reliability indexes for load conditions that do not include earthquake</p> <table border="1"> <thead> <tr> <th rowspan="2">Basis</th> <th colspan="4">Risk Category</th> </tr> <tr> <th>I</th> <th>II</th> <th>III</th> <th>IV</th> </tr> </thead> <tbody> <tr> <td>b1</td> <td>2.5</td> <td>3.0</td> <td>3.25</td> <td>3.5</td> </tr> <tr> <td>b2</td> <td>3.0</td> <td>3.5</td> <td>3.75</td> <td>4.0</td> </tr> <tr> <td>b3</td> <td>3.5</td> <td>4.0</td> <td>4.25</td> <td>4.5</td> </tr> </tbody> </table> <p>- b1: failure that is not sudden and does not lead to widespread progression of damage          - b2: failure that is either sudden or leads to widespread progression of damage          - b3: failure that is sudden and results in widespread progression of damage</p> <p>■ anticipated reliability (maximum probability of failure) for earthquake</p> <table border="1"> <thead> <tr> <th rowspan="2"></th> <th colspan="3">Risk Category</th> </tr> <tr> <th>I &amp; II</th> <th>III</th> <th>IV</th> </tr> </thead> <tbody> <tr> <td>f1</td> <td>10%</td> <td>6%</td> <td>3%</td> </tr> <tr> <td>f2</td> <td>25%</td> <td>15%</td> <td>10%</td> </tr> </tbody> </table> <p>- f1: total or partial structural collapse          - f2: failure that could result in endangerment of individual lives</p>	Basis	Risk Category				I	II	III	IV	b1	2.5	3.0	3.25	3.5	b2	3.0	3.5	3.75	4.0	b3	3.5	4.0	4.25	4.5		Risk Category			I & II	III	IV	f1	10%	6%	3%	f2	25%	15%	10%	<p>■ CAN/CSA-S6-06 : <math>\beta = 3.50</math>          - for bridges with a 75-year design life</p> <p>■ Bartlett et al. (2003) : <math>\beta \geq 3.0</math>          - normal building components with a 50-year design life for ductile failures</p> <p>■ CSA(1981)          - steel and concrete buildings for ULS based on 30-year life</p> <table border="1"> <thead> <tr> <th rowspan="2">Safety Class</th> <th colspan="2">Type of Failure</th> </tr> <tr> <th>Gradual</th> <th>Sudden</th> </tr> </thead> <tbody> <tr> <td>Not serious</td> <td>2.5</td> <td>3.0</td> </tr> <tr> <td>Serious (normal buildings)</td> <td>3.5</td> <td>4.0</td> </tr> <tr> <td>Very serious</td> <td>4.0</td> <td>4.5</td> </tr> </tbody> </table> <p>■ CSA S408-81 : <math>\beta=3.5</math>          for brittle failures with a ??-year design life</p> <p>■ Bartlett (2007) : <math>\beta=4.0</math>          for brittle failure of concrete element with a ??-year design life</p> <p>■ CISC 2010 : <math>\beta=4.5</math>          for fracture of steel on net section with a ??-year design life</p>	Safety Class	Type of Failure		Gradual	Sudden	Not serious	2.5	3.0	Serious (normal buildings)	3.5	4.0	Very serious	4.0	4.5	<p>■ Annual structural reliability indices (<math>\beta</math>) for structural components and connections</p> <table border="1"> <thead> <tr> <th>Importance Level</th> <th>Permanent &amp; imposed actions</th> <th>Wind, earthquake &amp; snow actions</th> </tr> </thead> <tbody> <tr> <td>1</td> <td></td> <td>3.2</td> </tr> <tr> <td>2</td> <td>3.8</td> <td>3.4</td> </tr> <tr> <td>3</td> <td></td> <td>3.6</td> </tr> <tr> <td>4</td> <td></td> <td>3.8</td> </tr> </tbody> </table>	Importance Level	Permanent & imposed actions	Wind, earthquake & snow actions	1		3.2	2	3.8	3.4	3		3.6	4		3.8
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