

| 章節 | ページ | 誤 | 正 | | | | | | | | | | | | | | | | | | | | | | | |
|-----------|------------------|--|--|--------------|------|-----|------|------------------|--------------|------------------|--------------|--|-----------|--------|--|--------|--|-----|------|-----|------|--|------------------|--------------|------------------|--------------|
| 2.1 | 18 上5行 | $q_u = \alpha \times (5.14 C_2) + \beta \times (\gamma_1 \times H_1)$ | $q_u = \alpha \times (5.14 C_2) + \gamma_1 \times H_1$ | | | | | | | | | | | | | | | | | | | | | | | |
| 2.1 | 18 上12行 | $q_u = 1.2 \times (5.14 \times 48.3) + 0.3 \times (18.6 \times 5.0)$ | $q_u = 1.2 \times (5.14 \times 48.3) + 18.6 \times 5.0$ | | | | | | | | | | | | | | | | | | | | | | | |
| 2.1 | 18 上13行 | $= 297.9 + 87.9 = 325.8 \text{ kN/m}^2$ | $= 297.9 + 93.0 = 390.9 \text{ kN/m}^2$ | | | | | | | | | | | | | | | | | | | | | | | |
| 2.2 | 34 | $c \cdot N_c$ が 408 kN/m^2 | $c \cdot N_c$ が 403 kN/m^2 | | | | | | | | | | | | | | | | | | | | | | | |
| 2.2 | 37 | 図 2.2.12 使用限界検討用の応力図 | 図 2.2.12 不同沈下による付加応力 | | | | | | | | | | | | | | | | | | | | | | | |
| 2.5 | 98 | 図 2.5.4 中の深度の欄 : 5.25 | 図 2.5.4 中の深度の欄 : 5.75 | | | | | | | | | | | | | | | | | | | | | | | |
| 2.5 | 101 | (最後の行) 右辺の分母の中括弧 $\{1 + \sqrt{\quad}\}$ | $\{1 + \sqrt{\quad}\}^2$ (計算はあっています) | | | | | | | | | | | | | | | | | | | | | | | |
| 2.5 | 115 | 参考文献 2002年3月号 | 2000年2月号 | | | | | | | | | | | | | | | | | | | | | | | |
| 3.2 | 149 | 表 3.2.10 地盤から定まる鉛直支持力(kN) 杭の鉛直支持力(kN) 終局 3 100 | 地盤から定まる鉛直支持力(kN) 杭の鉛直支持力(kN) 終局 3 140 | | | | | | | | | | | | | | | | | | | | | | | |
| 3.2 | 150 上8行 | ~13.85m では | ~13.95m では | | | | | | | | | | | | | | | | | | | | | | | |
| 3.2 | 160 上12行 | $J = (7/8)d = 402 \text{ mm}$ | $J = (7/8)d = 402.5 \text{ mm}$ | | | | | | | | | | | | | | | | | | | | | | | |
| 3.2 | 161 上4行 | $E_{杭} = 4.0 \times 10^5 \text{ N/mm}^2$ | $E_{杭} = 4.0 \times 10^4 \text{ N/mm}^2$ | | | | | | | | | | | | | | | | | | | | | | | |
| 3.2 | 161 下8行 | $8.0 + 0.958 \text{ N} / (209 \times 403)$ | $8.0 + 0.700 \text{ N} / (209 \times 402.5)$ | | | | | | | | | | | | | | | | | | | | | | | |
| 3.3 | 169 | (表 3.3.5) 液状化抵抗比 τ_d / σ'_z | 液状化抵抗比 τ_d / σ'_z | | | | | | | | | | | | | | | | | | | | | | | |
| 3.3 | 169 | 表 3.3.6 <table border="1" style="width: 100%; border-collapse: collapse; text-align: center;"> <tr> <td rowspan="2">深度 (m)</td> <td>繰返し</td> <td>水平変位</td> <td>繰返し</td> <td>水平変位</td> </tr> <tr> <td>せん断ひずみ cy (%)</td> <td>残留沈下 (mm)</td> <td>せん断ひずみ cy (%)</td> <td>残留沈下 (mm)</td> </tr> </table> | 深度 (m) | 繰返し | 水平変位 | 繰返し | 水平変位 | せん断ひずみ cy (%) | 残留沈下 (mm) | せん断ひずみ cy (%) | 残留沈下 (mm) | <table border="1" style="width: 100%; border-collapse: collapse; text-align: center;"> <tr> <td rowspan="2">深度 (m)</td> <td colspan="2">損傷限界状態</td> <td colspan="2">終局限界状態</td> </tr> <tr> <td>繰返し</td> <td>水平変位</td> <td>繰返し</td> <td>水平変位</td> </tr> <tr> <td></td> <td>せん断ひずみ cy (%)</td> <td>残留沈下 (mm)</td> <td>せん断ひずみ cy (%)</td> <td>残留沈下 (mm)</td> </tr> </table> | 深度 (m) | 損傷限界状態 | | 終局限界状態 | | 繰返し | 水平変位 | 繰返し | 水平変位 | | せん断ひずみ cy (%) | 残留沈下 (mm) | せん断ひずみ cy (%) | 残留沈下 (mm) |
| 深度 (m) | 繰返し | 水平変位 | | 繰返し | 水平変位 | | | | | | | | | | | | | | | | | | | | | |
| | せん断ひずみ cy (%) | 残留沈下 (mm) | せん断ひずみ cy (%) | 残留沈下 (mm) | | | | | | | | | | | | | | | | | | | | | | |
| 深度 (m) | 損傷限界状態 | | 終局限界状態 | | | | | | | | | | | | | | | | | | | | | | | |
| | 繰返し | 水平変位 | 繰返し | 水平変位 | | | | | | | | | | | | | | | | | | | | | | |
| | せん断ひずみ cy (%) | 残留沈下 (mm) | せん断ひずみ cy (%) | 残留沈下 (mm) | | | | | | | | | | | | | | | | | | | | | | |
| 3.3 | 171 | (表 3.3.9 下4行目) 杭体の許容応力度以下 | 杭体の長期許容応力度以下 | | | | | | | | | | | | | | | | | | | | | | | |
| 3.4 | 214 | (下7行目) (1)に示す | 3.4.4 に示す | | | | | | | | | | | | | | | | | | | | | | | |
| 3.6 | 244 | 表 3.6.5 支持地盤 内部摩擦角 ϕ (°) 0 | 【以下 P248 までは指針(3.4.3a)式の修正に伴う訂正】 支持地盤 内部摩擦角 ϕ (°) 5.5 | | | | | | | | | | | | | | | | | | | | | | | |

建築基礎構造設計例集正誤表

| 章節 | ページ | 誤 | 正 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
|--|--|--|--|--|-----------------------------------|-----------------------|---------------|--------------|--|-------------------------------------|--|-------------------------------|-----------------------|--|---|--|--|---|---|--------------|---------------|------------------|---|----------------|----------------|---------------|-------------------------------------|---------------|------|------|--|-------|---|--------------------------|---|-----------------------------------|-----|------|--------------|----------|-----------------|------------|------------------|-----------------------|-----------------|-------|-----|---|---|---|-----|-------|-----|-----|------|------|------|-------|-----|-----|------|------|------|
| 3.6 | 245 | <p>表 3.6.6</p> <table border="1"> <tr> <th>限界状態</th> <th>主働土圧係数 K_A, K_{EA}</th> <th>主働土圧の合力 P_A, P_{EA} (kN)</th> </tr> <tr> <td>使用限界状態</td> <td>0.35</td> <td>337</td> </tr> </table> <p>注)指針正誤表 3.4 P.26 参照</p> | 限界状態 | 主働土圧係数 K_A, K_{EA} | 主働土圧の合力 P_A, P_{EA} (kN) | 使用限界状態 | 0.35 | 337 | <table border="1"> <tr> <th>限界状態</th> <th>主働土圧係数 K_A, K_{EA}</th> <th>主働土圧の合力 P_A, P_{EA} (kN)</th> </tr> <tr> <td>使用限界状態</td> <td>0.38</td> <td>366</td> </tr> </table> | 限界状態 | 主働土圧係数 K_A, K_{EA} | 主働土圧の合力 P_A, P_{EA} (kN) | 使用限界状態 | 0.38 | 366 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| 限界状態 | 主働土圧係数 K_A, K_{EA} | 主働土圧の合力 P_A, P_{EA} (kN) | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| 使用限界状態 | 0.35 | 337 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| 限界状態 | 主働土圧係数 K_A, K_{EA} | 主働土圧の合力 P_A, P_{EA} (kN) | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| 使用限界状態 | 0.38 | 366 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| 3.6 | 245 | 主働土圧による転倒モーメント 430kNm | 主働土圧による転倒モーメント 466kNm | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| 3.6 | 246 | <p>表 3.6.7</p> <table border="1"> <thead> <tr> <th colspan="2">主働土圧による 転倒モーメント M_0</th> <th colspan="3">常時土圧 + 地震時慣性力 による転倒モーメント M_0</th> <th>と の 大 きい方</th> </tr> <tr> <th>n (m)</th> <th>M_0 (kN・m)</th> <th>H/2 (m)</th> <th>M_0' (kN・m)</th> <th>$M_0^{(1)}$ (kN・m)</th> <th>M_0 (kN・m)</th> </tr> </thead> <tbody> <tr> <td>1.275</td> <td>430</td> <td>-</td> <td>-</td> <td>-</td> <td>430</td> </tr> <tr> <td>1.275</td> <td>623</td> <td>5.0</td> <td>1006</td> <td>1436</td> <td>1436</td> </tr> <tr> <td>1.275</td> <td>698</td> <td>5.0</td> <td>1257</td> <td>1687</td> <td>1687</td> </tr> </tbody> </table> | 主働土圧による 転倒モーメント M_0 | | 常時土圧 + 地震時慣性力 による転倒モーメント M_0 | | | と の 大 きい方 | n (m) | M_0 (kN・m) | H/2 (m) | M_0' (kN・m) | $M_0^{(1)}$ (kN・m) | M_0 (kN・m) | 1.275 | 430 | - | - | - | 430 | 1.275 | 623 | 5.0 | 1006 | 1436 | 1436 | 1.275 | 698 | 5.0 | 1257 | 1687 | 1687 | <table border="1"> <thead> <tr> <th colspan="2">主働土圧による 転倒モーメント M_0</th> <th colspan="3">常時土圧 + 地震時慣性力 による転倒モーメント M_0</th> <th>と の 大 きい方</th> </tr> <tr> <th>n (m)</th> <th>M_0 (kN・m)</th> <th>H/2 (m)</th> <th>M_0' (kN・m)</th> <th>$M_0^{(1)}$ (kN・m)</th> <th>M_0 (kN・m)</th> </tr> </thead> <tbody> <tr> <td>1.275</td> <td>466</td> <td>-</td> <td>-</td> <td>-</td> <td>466</td> </tr> <tr> <td>1.275</td> <td>623</td> <td>5.0</td> <td>1006</td> <td>1472</td> <td>1472</td> </tr> <tr> <td>1.275</td> <td>698</td> <td>5.0</td> <td>1257</td> <td>1723</td> <td>1723</td> </tr> </tbody> </table> | 主働土圧による 転倒モーメント M_0 | | 常時土圧 + 地震時慣性力 による転倒モーメント M_0 | | | と の 大 きい方 | n (m) | M_0 (kN・m) | H/2 (m) | M_0' (kN・m) | $M_0^{(1)}$ (kN・m) | M_0 (kN・m) | 1.275 | 466 | - | - | - | 466 | 1.275 | 623 | 5.0 | 1006 | 1472 | 1472 | 1.275 | 698 | 5.0 | 1257 | 1723 | 1723 |
| 主働土圧による 転倒モーメント M_0 | | 常時土圧 + 地震時慣性力 による転倒モーメント M_0 | | | と の 大 きい方 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| n (m) | M_0 (kN・m) | H/2 (m) | M_0' (kN・m) | $M_0^{(1)}$ (kN・m) | M_0 (kN・m) | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| 1.275 | 430 | - | - | - | 430 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| 1.275 | 623 | 5.0 | 1006 | 1436 | 1436 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| 1.275 | 698 | 5.0 | 1257 | 1687 | 1687 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| 主働土圧による 転倒モーメント M_0 | | 常時土圧 + 地震時慣性力 による転倒モーメント M_0 | | | と の 大 きい方 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| n (m) | M_0 (kN・m) | H/2 (m) | M_0' (kN・m) | $M_0^{(1)}$ (kN・m) | M_0 (kN・m) | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| 1.275 | 466 | - | - | - | 466 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| 1.275 | 623 | 5.0 | 1006 | 1472 | 1472 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| 1.275 | 698 | 5.0 | 1257 | 1723 | 1723 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| 3.6 | 246 | <p>表 3.6.8</p> <table border="1"> <tr> <th>限界状態</th> <th>P_{AH} (kN)</th> </tr> <tr> <td>使用限界状態</td> <td>324</td> </tr> <tr> <td>損傷限界状態</td> <td>525</td> </tr> <tr> <td>終局限界状態</td> <td>576</td> </tr> </table> | 限界状態 | P_{AH} (kN) | 使用限界状態 | 324 | 損傷限界状態 | 525 | 終局限界状態 | 576 | <table border="1"> <tr> <th>限界状態</th> <th>P_{AH} (kN)</th> </tr> <tr> <td>使用限界状態</td> <td>351</td> </tr> <tr> <td>損傷限界状態</td> <td>553</td> </tr> <tr> <td>終局限界状態</td> <td>603</td> </tr> </table> | 限界状態 | P_{AH} (kN) | 使用限界状態 | 351 | 損傷限界状態 | 553 | 終局限界状態 | 603 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| 限界状態 | P_{AH} (kN) | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| 使用限界状態 | 324 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| 損傷限界状態 | 525 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| 終局限界状態 | 576 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| 限界状態 | P_{AH} (kN) | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| 使用限界状態 | 351 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| 損傷限界状態 | 553 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| 終局限界状態 | 603 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| 3.6 | 246 | <p>表 3.6.9</p> <table border="1"> <tr> <th>最大接地圧 σ_{max} (kN/m²)</th> <th>最小接地圧 σ_{min} (kN/m²)</th> <th>単位長さ当たりの 作用鉛直力 (kN/m)</th> </tr> <tr> <td>163</td> <td>151</td> <td>1085</td> </tr> <tr> <td>274</td> <td>40</td> <td>1085</td> </tr> <tr> <td>-</td> <td>-</td> <td>1085</td> </tr> </table> | 最大接地圧 σ_{max} (kN/m ²) | 最小接地圧 σ_{min} (kN/m ²) | 単位長さ当たりの 作用鉛直力 (kN/m) | 163 | 151 | 1085 | 274 | 40 | 1085 | - | - | 1085 | <table border="1"> <tr> <th>最大接地圧 σ_{max} (kN/m²)</th> <th>最小接地圧 σ_{min} (kN/m²)</th> <th>単位長さ当たりの 作用鉛直力 (kN/m)</th> </tr> <tr> <td>160</td> <td>156</td> <td>1106</td> </tr> <tr> <td>283</td> <td>33</td> <td>1106</td> </tr> <tr> <td>-</td> <td>-</td> <td>1106</td> </tr> </table> | 最大接地圧 σ_{max} (kN/m ²) | 最小接地圧 σ_{min} (kN/m ²) | 単位長さ当たりの 作用鉛直力 (kN/m) | 160 | 156 | 1106 | 283 | 33 | 1106 | - | - | 1106 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| 最大接地圧 σ_{max} (kN/m ²) | 最小接地圧 σ_{min} (kN/m ²) | 単位長さ当たりの 作用鉛直力 (kN/m) | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| 163 | 151 | 1085 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| 274 | 40 | 1085 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| - | - | 1085 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| 最大接地圧 σ_{max} (kN/m ²) | 最小接地圧 σ_{min} (kN/m ²) | 単位長さ当たりの 作用鉛直力 (kN/m) | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| 160 | 156 | 1106 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| 283 | 33 | 1106 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| - | - | 1106 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| 3.6 | 247 | <p>表 3.6.11</p> <table border="1"> <tr> <th>荷重傾斜角 (°)</th> <th>偏心距離 e (m)</th> <th>有効幅 B_e (m)</th> <th>支持力の限界値¹⁾</th> </tr> <tr> <td>16</td> <td>0.05</td> <td>6.9</td> <td>$q_y = (2/3)q_0 = 222\text{kN/m}^2$</td> </tr> <tr> <td>26</td> <td>0.87</td> <td>5.3</td> <td>$q_0 = 315\text{kN/m}^2$ $R_y = (2/3) q_0 B_e = 2/3 \times 315 \times 5.3 = 1,113\text{kN/m}^2$</td> </tr> <tr> <td>28</td> <td>1.10</td> <td>4.8</td> <td>$R_x = q_0 B_e = 239 \times 4.8 = 1,147\text{kN/m}$</td> </tr> </table> <p>¹⁾は設計例 2.5 の図 2.5.7 による。ただし R_x, R_y は擁壁の単位長さ当たりの値。</p> | 荷重傾斜角 (°) | 偏心距離 e (m) | 有効幅 B_e (m) | 支持力の限界値 ¹⁾ | 16 | 0.05 | 6.9 | $q_y = (2/3)q_0 = 222\text{kN/m}^2$ | 26 | 0.87 | 5.3 | $q_0 = 315\text{kN/m}^2$ $R_y = (2/3) q_0 B_e = 2/3 \times 315 \times 5.3 = 1,113\text{kN/m}^2$ | 28 | 1.10 | 4.8 | $R_x = q_0 B_e = 239 \times 4.8 = 1,147\text{kN/m}$ | <table border="1"> <tr> <th>荷重傾斜角 (°)</th> <th>偏心距離 e (m)</th> <th>有効幅 B_e (m)</th> <th>支持力の限界値¹⁾</th> </tr> <tr> <td>18</td> <td>0.01</td> <td>7.0</td> <td>$q_y = (2/3)q_0 = 283\text{kN/m}^2$</td> </tr> <tr> <td>27</td> <td>0.92</td> <td>5.2</td> <td>$q_0 = 326\text{kN/m}^2$ $R_y = (2/3) q_0 B_e = 2/3 \times 326 \times 5.2 = 1,130\text{kN/m}^2$</td> </tr> <tr> <td>29</td> <td>1.15</td> <td>4.7</td> <td>$R_x = q_0 B_e = 365 \times 4.7 = 1,433\text{kN/m}$</td> </tr> </table> <p>¹⁾は設計例 2.5 の図 2.5.7 による。ただし R_x, R_y は擁壁の単位長さ当たりの値。</p> | 荷重傾斜角 (°) | 偏心距離 e (m) | 有効幅 B_e (m) | 支持力の限界値 ¹⁾ | 18 | 0.01 | 7.0 | $q_y = (2/3)q_0 = 283\text{kN/m}^2$ | 27 | 0.92 | 5.2 | $q_0 = 326\text{kN/m}^2$ $R_y = (2/3) q_0 B_e = 2/3 \times 326 \times 5.2 = 1,130\text{kN/m}^2$ | 29 | 1.15 | 4.7 | $R_x = q_0 B_e = 365 \times 4.7 = 1,433\text{kN/m}$ | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| 荷重傾斜角 (°) | 偏心距離 e (m) | 有効幅 B_e (m) | 支持力の限界値 ¹⁾ | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| 16 | 0.05 | 6.9 | $q_y = (2/3)q_0 = 222\text{kN/m}^2$ | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| 26 | 0.87 | 5.3 | $q_0 = 315\text{kN/m}^2$ $R_y = (2/3) q_0 B_e = 2/3 \times 315 \times 5.3 = 1,113\text{kN/m}^2$ | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| 28 | 1.10 | 4.8 | $R_x = q_0 B_e = 239 \times 4.8 = 1,147\text{kN/m}$ | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| 荷重傾斜角 (°) | 偏心距離 e (m) | 有効幅 B_e (m) | 支持力の限界値 ¹⁾ | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| 18 | 0.01 | 7.0 | $q_y = (2/3)q_0 = 283\text{kN/m}^2$ | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
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| 29 | 1.15 | 4.7 | $R_x = q_0 B_e = 365 \times 4.7 = 1,433\text{kN/m}$ | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| 3.6 | 247 | <p>I点の沈下量/ 2</p> <p>□AIPH(q7)-□BJPH(q7-q6)-□CKPH (q6-q5)</p> <p>・・・□FNPH(q3-q2)-□GOPH(q2-q1)</p> | <p>□AIPH(q7)-□AIOG(q7-q6)-□AINF (q6-q5)</p> <p>・・・□AIKC(q3-q2)-□AIJB(q2-q1)</p> | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| 3.6 | 248 | <p>表 3.6.12</p> <table border="1"> <tr> <th>I 点沈下量 (mm)</th> <th>P 点沈下量 (mm)</th> <th>平均沈下量 (mm)</th> <th>基礎傾斜角 1/</th> <th>頂部変位量 (mm)</th> </tr> <tr> <td>35.6</td> <td>37.0</td> <td>36.4</td> <td>5137</td> <td>1.9</td> </tr> <tr> <td>23.1</td> <td>49.6</td> <td>36.4</td> <td>264</td> <td>37.9</td> </tr> <tr> <td>19.6</td> <td>53.1</td> <td>36.4</td> <td>209</td> <td>47.9</td> </tr> </table> | I 点沈下量 (mm) | P 点沈下量 (mm) | 平均沈下量 (mm) | 基礎傾斜角 1/ | 頂部変位量 (mm) | 35.6 | 37.0 | 36.4 | 5137 | 1.9 | 23.1 | 49.6 | 36.4 | 264 | 37.9 | 19.6 | 53.1 | 36.4 | 209 | 47.9 | <table border="1"> <tr> <th>I 点沈下量 (mm)</th> <th>P 点沈下量 (mm)</th> <th>平均沈下量 (mm)</th> <th>基礎傾斜角 1/</th> <th>頂部変位量 (mm)</th> </tr> <tr> <td>36.4</td> <td>36.8</td> <td>36.6</td> <td>17442</td> <td>0.6</td> </tr> <tr> <td>22.5</td> <td>50.8</td> <td>36.6</td> <td>247</td> <td>40.5</td> </tr> <tr> <td>19.0</td> <td>54.3</td> <td>36.6</td> <td>198</td> <td>50.4</td> </tr> </table> | I 点沈下量 (mm) | P 点沈下量 (mm) | 平均沈下量 (mm) | 基礎傾斜角 1/ | 頂部変位量 (mm) | 36.4 | 36.8 | 36.6 | 17442 | 0.6 | 22.5 | 50.8 | 36.6 | 247 | 40.5 | 19.0 | 54.3 | 36.6 | 198 | 50.4 | | | | | | | | | | | | | | | | | | | | |
| I 点沈下量 (mm) | P 点沈下量 (mm) | 平均沈下量 (mm) | 基礎傾斜角 1/ | 頂部変位量 (mm) | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| 35.6 | 37.0 | 36.4 | 5137 | 1.9 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| 23.1 | 49.6 | 36.4 | 264 | 37.9 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| 19.6 | 53.1 | 36.4 | 209 | 47.9 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| I 点沈下量 (mm) | P 点沈下量 (mm) | 平均沈下量 (mm) | 基礎傾斜角 1/ | 頂部変位量 (mm) | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| 36.4 | 36.8 | 36.6 | 17442 | 0.6 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| 22.5 | 50.8 | 36.6 | 247 | 40.5 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| 19.0 | 54.3 | 36.6 | 198 | 50.4 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| 3.7 | 276 上 2 行 | $f_c = 7.5\text{kN/mm}^2, f_s = 0.60\text{kN/mm}^2$ | $f_c = 7.5\text{N/mm}^2, f_s = 0.60\text{N/mm}^2$ | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |

建築基礎構造設計例集正誤表

| 章節 | ページ | 誤 | 正 | | | | | | | | | | | | | | | | | | | | |
|----------|-------------|---|--|--|----|-----|-------|-------|-------|-------|---|--|---|----------|--|----|-----|-------|-------|-------|-------|---|--|
| 3.7 | 276 下10行 | $f_a=215\text{kN/mm}^2$ | $f_a=215\text{N/mm}^2$ | | | | | | | | | | | | | | | | | | | | |
| 3.7 | 276 下7行 | $M_d=4363\text{kNm}$ | $M_d=4016\text{kNm}$ | | | | | | | | | | | | | | | | | | | | |
| 3.7 | p277 | (下から3行目) $N_d=2\sim 13\ 350\text{kN}$, $M_d=11\ 460\text{kNm}$ | $N_d=2\sim 13\ 960\text{kN}$, $M_d=11\ 350\text{kNm}$ | | | | | | | | | | | | | | | | | | | | |
| 3.8 | 287 | 表3. 8. 2 <table border="1" style="margin-left: auto; margin-right: auto;"> <thead> <tr> <th colspan="2">積載荷重(kN)</th> </tr> <tr> <th>常時</th> <th>地震時</th> </tr> </thead> <tbody> <tr> <td>33260</td> <td>14180</td> </tr> <tr> <td>32030</td> <td>22810</td> </tr> <tr> <td colspan="2" style="text-align: center;">-</td> </tr> </tbody> </table> | 積載荷重(kN) | | 常時 | 地震時 | 33260 | 14180 | 32030 | 22810 | - | | <table border="1" style="margin-left: auto; margin-right: auto;"> <thead> <tr> <th colspan="2">積載荷重(kN)</th> </tr> <tr> <th>常時</th> <th>地震時</th> </tr> </thead> <tbody> <tr> <td>33220</td> <td>14220</td> </tr> <tr> <td>32070</td> <td>22820</td> </tr> <tr> <td colspan="2" style="text-align: center;">-</td> </tr> </tbody> </table> | 積載荷重(kN) | | 常時 | 地震時 | 33220 | 14220 | 32070 | 22820 | - | |
| 積載荷重(kN) | | | | | | | | | | | | | | | | | | | | | | | |
| 常時 | 地震時 | | | | | | | | | | | | | | | | | | | | | | |
| 33260 | 14180 | | | | | | | | | | | | | | | | | | | | | | |
| 32030 | 22810 | | | | | | | | | | | | | | | | | | | | | | |
| - | | | | | | | | | | | | | | | | | | | | | | | |
| 積載荷重(kN) | | | | | | | | | | | | | | | | | | | | | | | |
| 常時 | 地震時 | | | | | | | | | | | | | | | | | | | | | | |
| 33220 | 14220 | | | | | | | | | | | | | | | | | | | | | | |
| 32070 | 22820 | | | | | | | | | | | | | | | | | | | | | | |
| - | | | | | | | | | | | | | | | | | | | | | | | |
| 3.8 | 294 | 図3. 8. 9 ・杭頭部配筋は杭上端レベルから 11240mmの範囲とする。 | ・杭頭部配筋は杭上端レベルから 12600mmの範囲とする。 | | | | | | | | | | | | | | | | | | | | |
| 3.8 | 295 | 図3. 8. 10 東西断面 通り名: Y4b | Y4a | | | | | | | | | | | | | | | | | | | | |
| 3.8 | 295 | 図3. 8. 10 東西断面 通り名: Y0a (右側) | Y0 | | | | | | | | | | | | | | | | | | | | |